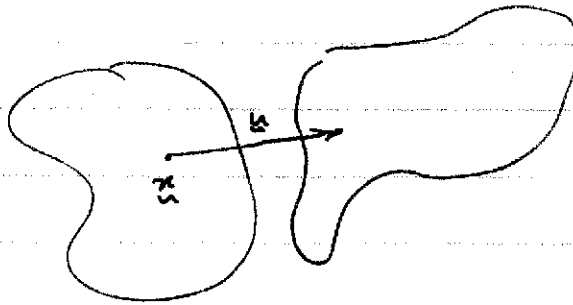


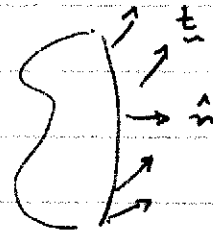
## 2. Elastodynamics

Kinematics

Displacement  $\underline{u}(\underline{x}, t)$ Strain  $\underline{\underline{\epsilon}} = \frac{1}{2} (\nabla \underline{u} + \nabla \underline{u}^T)$ Particle velocity  $\underline{v} = \frac{\partial \underline{u}}{\partial t}$ 

Balance laws.

1. Contact force  $\underline{t}(\hat{n}, \underline{x}, t)$
2. Body force  $\underline{f}(\underline{x}, t)$

Euler's postulate. For any part of the body  $P \subset \Omega$ 

$$\frac{d}{dt} \int_P \rho \underline{\underline{\dot{u}}} dV = \int_{\partial P} \underline{t} da + \int_P \underline{f} dV$$

$$\frac{d}{dt} \int_P \underline{x} \times \rho \underline{\underline{\dot{u}}} dV = \int_{\partial P} \underline{x} \times \underline{t} da + \int_P \underline{x} \times \underline{f} dV$$

$$\Rightarrow \underline{t}(\hat{n}, \underline{x}, t) = \underline{T}(\underline{x}, t) \hat{n}$$

$$\underline{T} = \underline{T}^T \quad \wedge \text{ Cauchy stress}$$

$$\rho \underline{\underline{\dot{u}}} = \text{div } \underline{T} + \underline{f}$$

Constitutive Relation  $\underline{T} = \underline{C} \underline{\epsilon}$

isotropic:  $T_{ij} = \lambda \epsilon_{kk} \delta_{ij} + 2\mu \epsilon_{ij}$

$\Rightarrow \rho \ddot{u} = \text{div } \underline{C} \nabla u$  if  $f = 0$

$$\rho \ddot{u}_i = \mu \Delta u_i + \frac{\mu}{1-2\nu} \nabla (\text{div } u) \quad \text{if isotropic}$$

$$\rho \ddot{u}_i = \mu u_{i,jj} + \frac{\mu}{1-2\nu} u_{j,ji}$$

Cauchy - Navier Equations.

Plane waves in an infinite homogeneous isotropic elastic medium.

A plane wave is a displacement field of the form

$$\underline{u}(\underline{x}, t) = \underline{a} f(\underline{x} \cdot \hat{m} - ct)$$

$\underline{a}$  constant vector

$\hat{m}$  unit vector

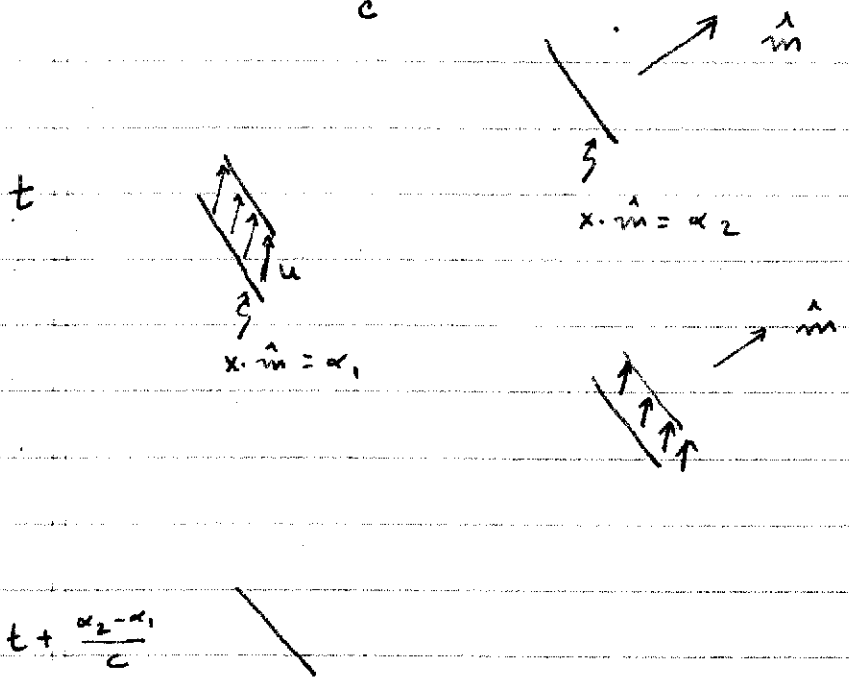
$f$  scalar function of one variable

$$u_i(\underline{x}, t) = a_i f(\underline{x} \cdot \hat{m} - ct)$$

Note 1. The displacement is always parallel to a

2. For any given time  $t$  and any  $\alpha$ , the displacement is constant on the plane  $\{x \cdot \hat{m} = \alpha\}$

3. ~~The displacement~~ For any given  $\alpha_2 > \alpha_1$ , the displacement of the plane  $\{x \cdot \hat{m} = \alpha_1\}$  at time  $t$  is exactly the same as the displacement of the plane  $\{x \cdot \hat{m} = \alpha_2\}$  at time  $t + \frac{\alpha_2 - \alpha_1}{c}$



So, the wave propagates in the direction  $\hat{m}$  at ~~the~~ speed  $c$ .

$$\begin{aligned} \nabla u : \quad u_{i,j} &= a_i m_j f'(x \cdot \hat{m} - ct) \\ \text{div } u = \text{tr } \underline{\underline{\varepsilon}} : \quad u_{i,i} &= a_i m_i f'(x \cdot \hat{m} - ct) \\ \text{curl } u : \quad \varepsilon_{ijk} u_{j,k} &= \varepsilon_{ijk} a_j m_k f'(x \cdot \hat{m} - ct) \\ \Delta u : \quad u_{i,jj} &= a_i f''(x \cdot \hat{m} - ct) \\ \nabla(\text{div } u) : \quad u_{jji} &= a_j m_j m_i f''(x \cdot \hat{m} - ct) \\ \dot{u} : \quad \dot{u}_i &= -c a_i f'(x \cdot \hat{m} - ct) \\ \ddot{u} : \quad \ddot{u}_i &= c^2 a_i f''(x \cdot \hat{m} - ct) \end{aligned}$$

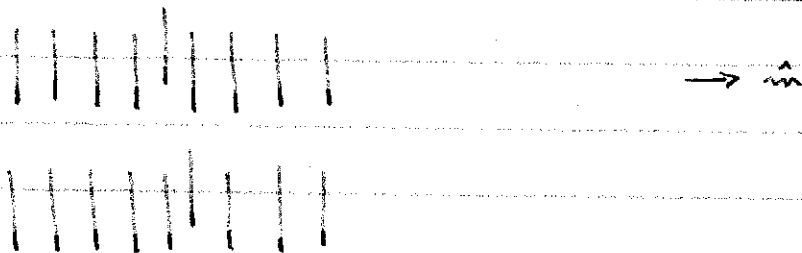
$\therefore$  Cauchy - Navier Equation

$$3c^2 a_i f'' = \mu a_i f'' + \frac{\mu}{1-2\nu} a_j m_j m_i f''$$

Assuming that  $f'' \neq 0$

$$(\mu - 3c^2) \underline{a} + \frac{\mu}{1-2\nu} (\underline{a} \cdot \hat{m}) \hat{m} = 0$$

$$\Rightarrow \text{Either } \underline{(\underline{a} \cdot \hat{m})} = 0, \quad \mu = 3c^2 \quad \text{or} \quad \boxed{\frac{c^2}{3} = \frac{\mu}{3}}$$



$\therefore$  purely shear deformation

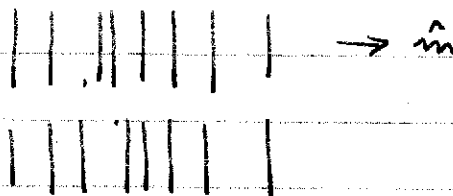
$$\text{vol. strain} = \text{tr } \underline{\underline{\varepsilon}} = \underline{a} \cdot \hat{m} f'' = 0.$$

These are called shear, rotational, transverse, secondary or S-waves.

or (ii)  $a \parallel \hat{m}$  or  $a = (a \cdot \hat{m}) \hat{m}$

$$\therefore \mu - 3c^2 + \frac{\mu}{1-2\nu} = 0$$

$$\text{or } c_p^2 = \frac{2(1-\nu)}{1-2\nu} \frac{\mu}{\rho} = \frac{\lambda+2\mu}{\rho} = \frac{3\mu}{\rho}$$



$$\text{curl } u = \underline{a} \times \hat{m} f'' = 0$$

... rotational ~~no~~ deformation.

These are called pressure, volumetric, longitudinal primary or P-wave.

Note:  $\frac{c_p^2}{c_s^2} = \frac{2(1-\nu)}{1-2\nu} \geq 1$  for  $\nu \in (0, \frac{1}{2})$ .

$\therefore$  P-waves travel faster than S-waves.

Summary: One can have only two types of plane waves in isotropic elastic media:

Longitudinal P-waves travelling at  $c_p$  or  
Transverse S-waves travelling at  $c_s$ .

No mixed waves.

-93-

## Some comments concerning the Cauchy-Navier Equations of elastodynamics

Let  $\underline{w} = \text{curl } \underline{u}$ ,  $\theta = \text{tr } \underline{\epsilon} = \text{div } \underline{u}$

$$(i) \quad \underline{w} = 0 \quad \Rightarrow \quad \underline{u}_{tt} = c_p^2 \Delta \underline{u}$$

$$(ii) \quad \theta = 0 \quad \Rightarrow \quad \underline{u}_{tt} = c_s^2 \Delta \underline{u}$$

$$(iii) \quad \theta_{tt} = c_p^2 \Delta \theta$$

$$(iv) \quad \underline{w}_{tt} = c_s^2 \Delta \underline{w}$$

Green - Lamé Solution.  $\underline{u}$  is a solution of the Cauchy-Navier equation if and only if

$$\underline{u} = \nabla \varphi + \text{curl } \underline{v}$$

where  $\varphi, \underline{v}$  satisfy

$$\varphi_{tt} = c_p^2 \Delta \varphi, \quad \underline{v}_{tt} = c_s^2 \Delta \underline{v}, \quad \text{div } \underline{v} = 0.$$

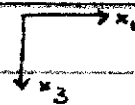
## Rayleigh Surface Waves

Consider a half space  $\{x_3 > 0\}$

with a traction free surface.

We seek a wave-like solution

of the Cauchy Navier equation that propagates in the  $x_1$ -direction and whose amplitude decays in the  $x_3$ -direction.



Thus we seek a solution to the Cauchy-Navier equation such that

1.  $u_2 = 0, \quad u_{2,i} = 0.$
2.  $u_i \rightarrow 0$  as  $x_3 \rightarrow \infty$
3.  $\frac{\partial}{\partial x_2} = 0$  on  $\{x_3 = 0\}$  i.e.,  $T_{3i} = 0$  on  $\{x_3 = 0\}$

Recalling the Green-Helmholtz solution and in accordance to (1):  $\varphi = \varphi(x_1, x_3, t)$

$$v = \begin{pmatrix} 0 \\ \psi(x_1, x_3, t) \\ 0 \end{pmatrix}$$

where  $\varphi_{tt} = c_p^2 (\varphi_{,11} + \varphi_{,33})$

$$\psi_{tt} = c_s^2 (\psi_{,11} + \psi_{,33})$$

Rayleigh ansatz:  $\varphi = f(x_3) \exp(i k (x_1 - c t))$   $c, k$  arbitrary constants

$$\psi = g(x_3) \exp(i k (x_1 - c t))$$

$$\Rightarrow f'' - k^2 \left(1 - \frac{c^2}{c_p^2}\right) f, \quad g'' - k^2 \left(1 - \frac{c^2}{c_s^2}\right) g$$

$$(2) \Rightarrow 0 < c < c_s \quad \text{since } c_s < c_p$$

$\Rightarrow$

$$f = A \exp(-\kappa_1 x_3), \quad g = B \exp(-\kappa_2 x_3)$$

$A, B$  arbitrary complex constants

$$\kappa_1 = k \sqrt{1 - \frac{c^2}{c_p^2}} > 0, \quad \kappa_2 = k \sqrt{1 - \frac{c^2}{c_s^2}} > 0$$

-95-

Recalling  $\underline{u} = \nabla\phi + \text{curl } \psi = \begin{pmatrix} \phi_{,1} - \psi_{,3} \\ 0 \\ \phi_{,3} + \psi_{,1} \end{pmatrix}$

$$u_1 = (ikA \exp(-\kappa_1 x_3) + \kappa_2 B \exp(-\kappa_2 x_3)) \exp(ik(x_1 - ct))$$

$$u_3 = (-\kappa_1 A \exp(-\kappa_1 x_3) + ikB \exp(-\kappa_2 x_3)) \exp(ik(x_1 - ct))$$

We seek to find  $A, B, c, k$  from (3).

Recall,

$$T_{31} = \mu(u_{3,1} + u_{1,3}), \quad T_{32} = 0,$$

$$T_{33} = \lambda(u_{,11} + u_{3,3}) + 2\mu u_{3,3}$$

$$(3) \Rightarrow \begin{cases} 2ik\kappa_1 A + \left(2 - \frac{c^2}{c_s^2}\right) k^2 B = 0 \\ \left(2 - \frac{c^2}{c_s^2}\right) ikA + 2\kappa_2 B = 0 \end{cases}$$

Note that these relations are independent of  $k$ .

To have a non-trivial solution,

$$\left(2 - \frac{c^2}{c_s^2}\right)^2 - 4 \sqrt{\left(1 - \frac{c^2}{c_p^2}\right)} \sqrt{\left(1 - \frac{c^2}{c_s^2}\right)} = 0$$

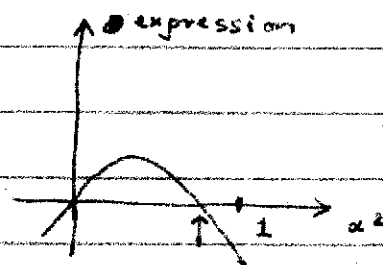
$$\text{Set } \alpha = \frac{c}{c_s}, \quad \beta = \frac{c_s}{c_p}$$

$$(2 - \alpha^2)^2 - 4 \sqrt{(1 - \beta^2 \alpha^2)} \sqrt{(1 - \alpha^2)} = 0$$

Exactly one root  $\alpha \in (0, 1)$ .

Depends on  $\beta$  or on  $\nu$

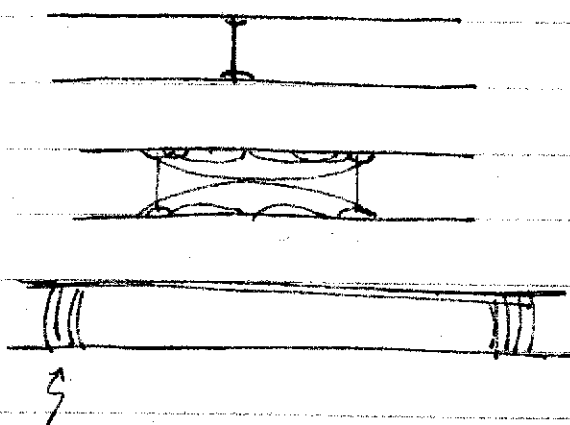
Can now find  $A/B$



### 3. Wave guide

Consider a bar as a three-dimensional elastic solid. Note that the bar wave speed,  $c_0 = \sqrt{\frac{E}{\rho}}$  is distinct from the longitudinal wave speed  $c_p$ . In fact one can verify that longitudinal plane waves do not satisfy the stress-free lateral boundary conditions ~~of a bar~~. ~~Conversely,~~ Conversely, one can show that bar waves do not solve the three-dimensional equations of elastodynamics.

Bar waves arise because bars act as "wave guides". An initial pulse is



large number of wave fronts

reflected numerous times from the boundary and at long times one only sees a cluster of wave fronts. Each wave front propagates according to 3-d

elastodynamics, but the cluster propagates differently.

-97-

Dispersive waves in one (space) dimension

Suppose we have a linear equation

$$P\left(\frac{\partial}{\partial x}, \frac{\partial}{\partial t}\right) \varphi = 0$$

where  $P(\bar{x}, T)$  is some polynomial

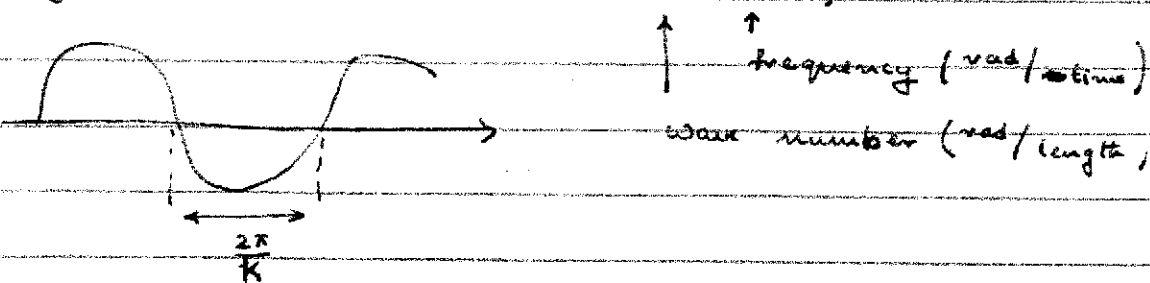
Eg: 1.  $P(\bar{x}, T) = \bar{x}^2 - \frac{1}{c^2} T^2$  in simple wave eqn.

2.  $P(\bar{x}, T) = EI \bar{x}^4 + SAT^2$  in ~~eq~~ for the deflection  
 of a small deflection  
 Euler-Bernoulli beam.

3.  $P(\bar{x}, T) = T^2 - c_0^2 \bar{x}^2 - \frac{1}{2} v^2 a^2 \bar{x}^2 T^2$  for a  
 solid bar  
 with round  
 cross-section.



Try:  $\varphi(x, t) = A \exp(i(kx - \omega t))$



Substituting in the equation,  $P(ik, -i\omega) = 0$

Solve:  $\omega = \omega_l(k)$   $l = 1, \dots, n$

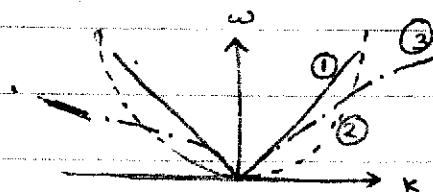
"Dispersion relation"

↑  
 depends on  
 order of polynomial

Eg: 1.  $c^2 k^2 = \omega^2$  or  $\omega = \pm ck$

2.  $\gamma^2 k^4 = \omega^2$  or  $\omega = \pm \gamma k^2$

||  
 $\frac{EI}{SA}$



3.  $\omega^2 = c_0^2 k^2 - \frac{1}{2} a^2 k^2 \omega^2 k^2$  or  $\omega = \pm \frac{k}{\sqrt{1 + \frac{1}{2} a^2 \gamma^2 k^2}}$

(Note that ~~the~~ bar equation is good for waves with small wave numbers; i.e., long wave lengths)

Terminology:

Phase:  $\theta(x, t) = kx - \omega t$

Instantaneous wave number

$$\frac{\partial \theta}{\partial x}$$

Inst. frequency

$$-\frac{\partial \theta}{\partial t}$$

Points of constant phase }  
propagate with velocity: }

$$\theta(x(t), t) = \theta_0$$

$$\therefore \theta_x \dot{x} + \theta_t = 0 \Rightarrow \dot{x} = \frac{-\theta_t}{\theta_x} = \frac{\omega}{k}$$

$$\Rightarrow \boxed{c_{\text{phase}} = \frac{\omega}{k} \quad \text{"phase velocity"}}$$

Velocity of points of constant wave number }

$$k(x(t), t) = k_0$$

$$k_x \dot{x} + k_t = 0$$

$$\theta_{xt} = -\omega_x = \frac{d\omega}{dk} k_x$$

$$\therefore k_x \dot{x} - \frac{d\omega}{dk} k_x = 0 \Rightarrow \dot{x} = \frac{d\omega}{dk}$$

- 99 -

$$\Rightarrow c_{\text{group}} = \frac{d\omega}{dk} \quad \text{"group velocity"}$$

It can be shown that

- wave packets

- energy

propagate with the group velocity.

Wave packets

$$u(x,t) = \int_{-\infty}^{\infty} g(k) \exp(i(kx - \omega(k)t)) dk$$

Suppose the waves are clustered near a wave number  $k_0$ .

$$\omega(k) = \omega(k_0) + \frac{d\omega}{dk}(k_0)(k-k_0) + \frac{1}{2} \frac{d^2\omega}{dk^2}(k_0)(k-k_0)^2 + \dots$$

If  $\frac{d^2\omega}{dk^2}(k_0)(k-k_0)^2 t \ll 1$ , then

$$u(x,t) \approx \int_{-\infty}^{\infty} g(k) \exp\left(i(k_0 x - \omega(k_0)t) + i\left((k-k_0) - \frac{d\omega}{dk}(k_0)(k-k_0)\right)\right) dk$$

$$= \underbrace{\exp(i(k_0 x - \omega(k_0)t))}_{\uparrow} \underbrace{\int_{-\infty}^{\infty} g(k) \exp\left(i\left((k-k_0) - \frac{d\omega}{dk}(k_0)(k-k_0)\right)\right) dk}_{\text{modulated by a wave}}$$

wave with velocity  $\frac{\omega(k_0)}{k_0}$

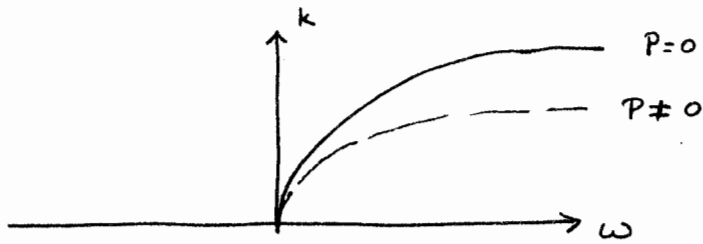
modulated by a wave  
with velocity  $\frac{d\omega}{dk}(k_0)$

and wave number  $k_0$

$$\exp(i(kx - \omega t))$$

$$\Rightarrow SIk^2\omega^2 = EI k^4 + \cancel{k^2 P} - SA\omega^2$$

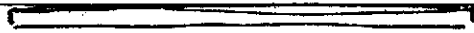
$$\omega^2 (SIk^2 + SA) = EI k^4 + Pk^2 \quad \text{or} \quad \omega^2 = \frac{EI k^4 + Pk^2}{SA + SIk^2}$$



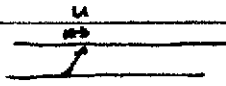
$P < 0$  ... not  
well posed  
for small  $k$

# Stress Waves

## 1. Waves in bars.



Bars are rods capable of sustaining only uniaxial stress and ~~uniaxial~~ longitudinal motion.

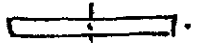
Kinematics: Displacement  $u(x,t)$  

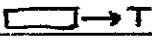
$$\text{Strain } \epsilon = \frac{\partial u}{\partial x} = u' = u_x$$

$$\text{Particle velocity } v = \frac{\partial u}{\partial t} = \dot{u} = u_t$$

$$\text{Compatibility: } v_x = \epsilon_t$$

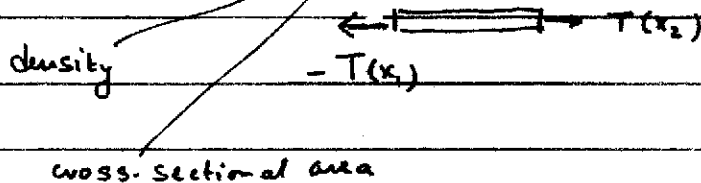
## Balance of linear momentum

~~Forces~~ Forces: 1. Contact force  $T(x)$  

2. Body force  $f(x)$    
per unit length

Euler's postulate: The rate of change of the momentum of any portion of a bar is equal to the external forces acting on it. Consider a portion  $(x_1, x_2)$ :

$$\frac{d}{dt} \int_{x_1}^{x_2} \rho A \dot{u} dx = T(x_2) - T(x_1) + \int_{x_1}^{x_2} f dx$$



$$\therefore \int_{x_1}^{x_2} SA \ddot{u} dx = \int_{x_1}^{x_2} \left( \frac{\partial T}{\partial x} + f \right) dx$$

$$\Rightarrow \boxed{SA \ddot{u} = \frac{\partial T}{\partial x} + f}$$

Constitutive relation:  $T = EA \epsilon = EA u'' = EA u_{xx}$

$\therefore$  Equation of a linear elastic bar  
with no body force:

$$S \ddot{u} = E u'' \quad \text{or} \quad S u_{tt} = E u_{xx}$$

$$\text{or} \quad \boxed{u_{tt} = c^2 u_{xx}} \quad c^2 = \frac{E}{S}$$

... wave equation.

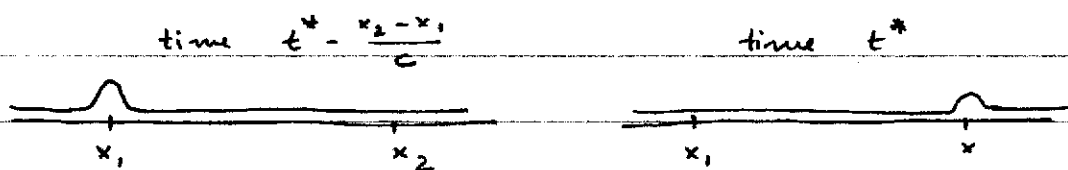
The solution to this equation can be  
written as:

$$u(x, t) = f(x - ct) + g(x + ct)$$

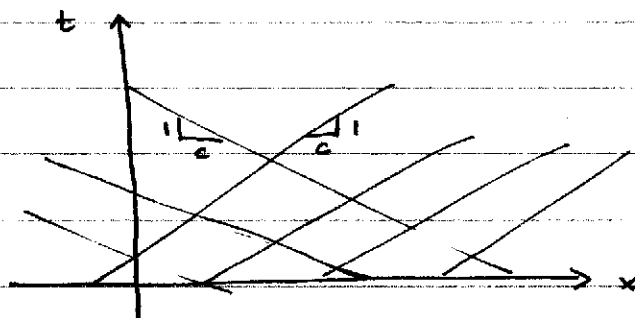
where  $f, g$  are smooth (twice cont. diff.)  
functions.

... superposition of a forward  
moving wave of velocity  $c$   
and a backward moving  
wave of velocity  $c$ .

Let  $x_1 < x_2$ . If  $u(x, t) = f(x - ct)$ , then the displacement at  $x_2$  at time  $t^*$  is exactly the same as the displacement at  $x_1$  at time  $t^* - \frac{x_2 - x_1}{c}$ . Therefore, the displacement moves forward in an undistorted wave at  $\frac{1}{c}$  speed  $c$ .



$\therefore f(x - ct)$  is a forward moving wave  
 $g(x + ct)$  is a backward moving wave.



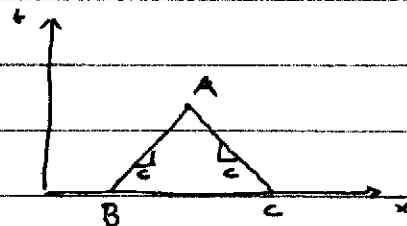
Note: Displacement (information) is propagated forward or backward at speed  $c$ ; ~~it is~~ or ~~it is~~ it is propagated along lines

$$x - ct = \text{const}$$

$$x + ct = \text{const}$$

... "characteristics".

Note: The displacement at A can only be affected by "events" in ABC. It is completely oblivious to events outside.



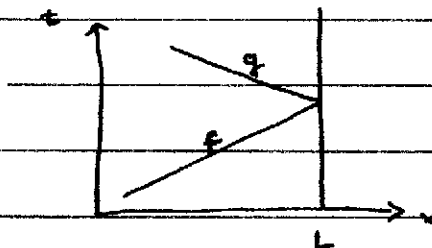
Boundary conditions

a) rigid boundary

$$u(L, t) = 0$$

$$\therefore f(L-ct) + g(L+ct) = 0$$

$$\text{or } f(L-ct) = -g(L+ct) \quad \forall t.$$



$\therefore$  ~~a wave~~ a forward moving wave is bounced off ~~it~~ with opposite displacement.

However,

$$+f'(L-ct) = +g'(L+ct)$$

$\therefore$  the strain of the forward moving wave ~~is~~ has the same sign as that of the backward moving wave:

tensile in  $\Rightarrow$  tensile back

comp. in  $\Rightarrow$  comp. back

b) free boundary :  $T(L, t) = 0 \Rightarrow \epsilon(L, t) = 0$

$$\therefore f'(L-ct) + g'(L+ct) = 0$$

$$\therefore f'(L-ct) = -g'(L+ct)$$

or a forward moving wave is bounced off with strain of the opposite sign.

tensile in  $\Rightarrow$  compressive back

comp. in  $\Rightarrow$  tensile back

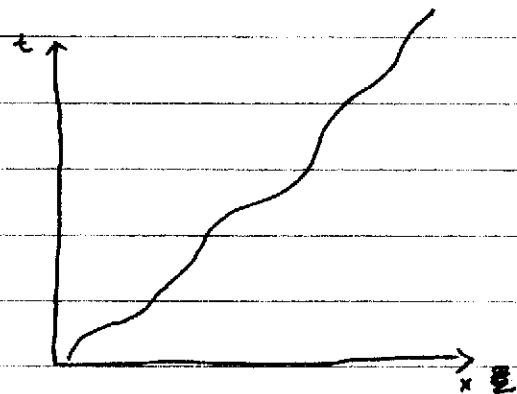
Also,  $\int f(L-ct) = \int g(L+ct) + \text{const}$

$\therefore$  a forward moving wave is bounced back with its displacement uniformly shifted (the shift depends on the initial data).

Shocks Solutions where the strain and particle velocity are discontinuous (across moving boundaries) but the displacement is continuous. Then we have to

replace the governing equations with "jump conditions"

Suppose we have a shock propagating at  $x = s(t)$



Displacement cont:  $u(s^+(t), t) = u(s^-(t), t)$

Differentiate with respect to  $t$ .

$$u_x(s^+(t), t) \dot{s} + u_t(s^+(t), t) = u_x(s^-(t), t) + u_t(s^-(t), t)$$

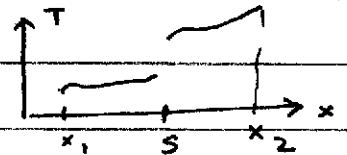
$$\textcircled{1} \quad \text{or} \quad \boxed{[v] + \dot{s} [\epsilon] = 0} \quad [q] = q(s^+) - q(s^-)$$

Balance of linear momentum:

$$\frac{d}{dt} \left[ \int_{x_1}^{s^-(t)} SA u_x dx + \int_{s^+(t)}^{x_2} SA u_x dx \right] = T(x_2) - T(x_1)$$

$$\therefore \int_{x_1}^{s^-} SA u_{xt} dx + \dot{s} SA u_t(s^-, t)$$

$$+ \int_{s^+}^{x_2} SA u_{xt} dx - \dot{s} SA u_t(s^+, t) = \int_{x_1}^{s^-} \frac{\partial T}{\partial x} dx + \int_{s^+}^{x_2} \frac{\partial T}{\partial x} dx - T(s^-, t) + T(s^+, t)$$



$$\Rightarrow \boxed{[T] + \dot{s} [SA v] = 0}$$

Since  $T = EA \epsilon$ ,

$$\textcircled{2} \quad \boxed{[EA \epsilon] + \dot{s} [SA v] = 0}$$

$\textcircled{1} + \textcircled{2}$  are the jump conditions.

Shocks in uniform bars:  $\rho, A, E$  continuous.

$$\therefore \textcircled{2} \Rightarrow E[\epsilon] + \rho s [v] = 0$$

$$\therefore \textcircled{1} + \textcircled{2} \Rightarrow \rho s^2 [\epsilon] - E[\epsilon] = 0$$

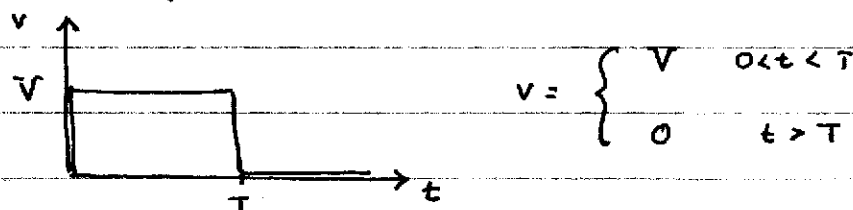
$$\Rightarrow \rho s^2 = E \quad \text{or} \quad \boxed{s^2 = \frac{E}{\rho} = c^2}$$

$\therefore$  Shocks propagate with the wave speed.

$\therefore$  we can take  $f, g$  to be continuous but with discontinuous first derivatives and enforce the jump conditions.

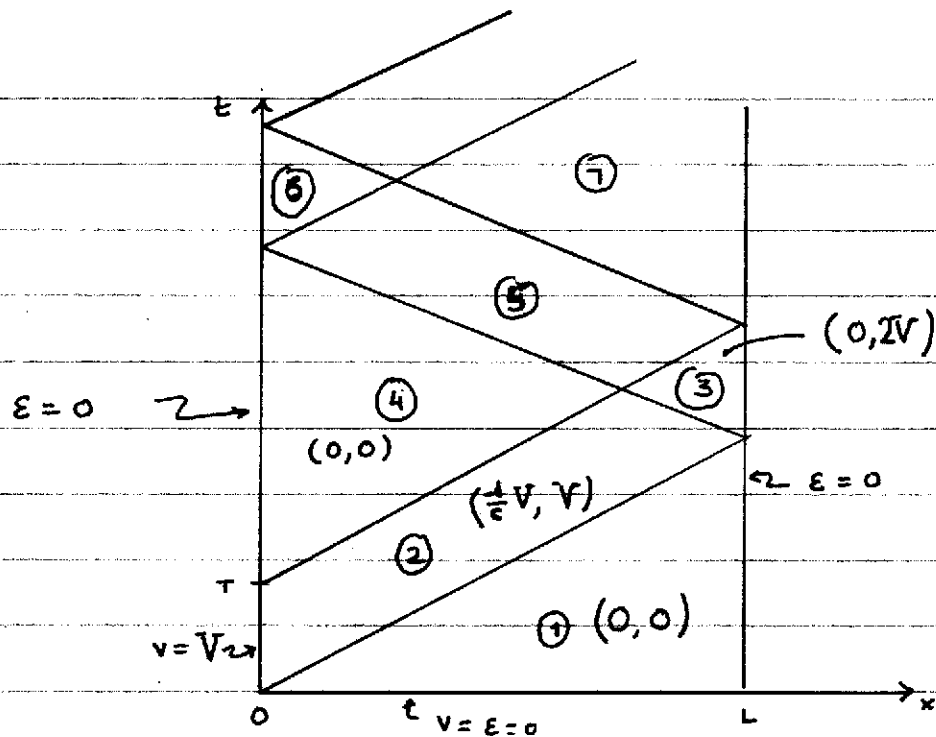
### Example 1. Impact problem.

Consider a bar of length  $L$  initially at rest with no applied forces, subjected to a velocity pulse at  $x=0$  as shown.



Since the initial and boundary conditions are piecewise constant <sup>(in  $v, \epsilon$ )</sup>, we look for piecewise constant solutions.

By looking at the characteristics, ~~we~~ we infer that it has the form shown in the figure.



Region 1:  $v = \varepsilon = 0$

Region 2:  $v = V$  (from b.c at  $x=0$ )

To find  $\varepsilon$ , look at the jump condition at the  $\textcircled{1}, \textcircled{2}$  boundary:

$$[v] + s[\varepsilon] = 0$$

$$(0 - V) + c(0 - \varepsilon) = 0 \Rightarrow \varepsilon = -\frac{1}{c} V$$

Region 3:  $\varepsilon = 0$

We can find  $v$  two ways:

First, jump condition at  $\textcircled{2}, \textcircled{3}$  boundary

$$(v - V) - c(0 - (-\frac{1}{c}V)) = 0$$

$$\Rightarrow v = 2V$$

Alternately, from our study of free boundary conditions, we know the "returning wave" has the opposite strain and same particle velocity. So region (5) is given by  $(\frac{1}{2}V, V)$

Region (3) is a superposition of regions (2) and (5). So in region (3)

$$(E, V) = \left(-\frac{1}{2}V, V\right) + \left(\frac{1}{2}V, V\right) = (0, 2V)$$

Region 4:  $v = E = 0$

either by inspection of lag a combination of b.c and (2)/(4) j.c.

Region 5:  $(E, V) = \left(\frac{1}{2}V, V\right)$

using the argument above, or using the (4)/(5) and (3)/(5) j.c.

#### Exercises

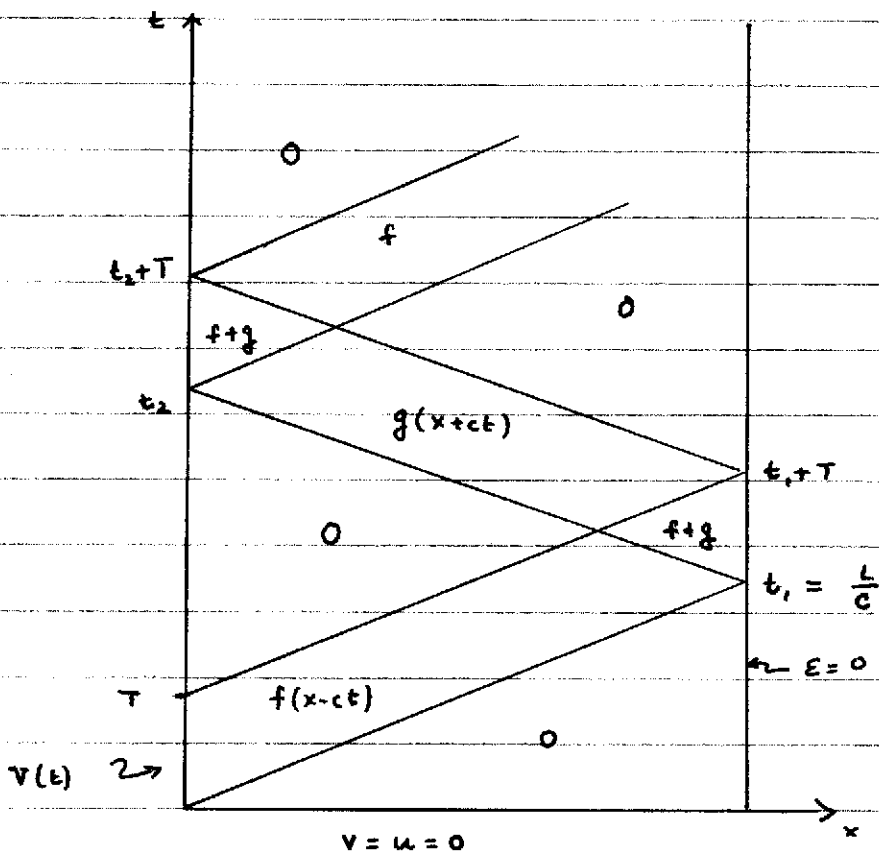
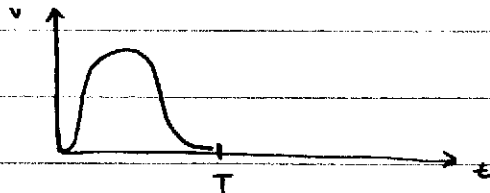
(a) Plot the <sup>total</sup> linear momentum as a function of time

(b) Plot the total kinetic energy as a function of time  $K.E = SA \int_0^L \frac{1}{2} v^2 dx$

(c) Plot the total potential energy  $P.E = SE \int_0^L \frac{1}{2} E^2 dx$

(d) Plot the total energy  $K.E + P.E$

Example 2 Same as example 1, but with general pulse



We have to find  $f, g$  on the intervals  $(-cT, 0)$ ,  $(ct, ct+T)$  resp.

At  $x=0$ ,  $\dot{u}(0, t) = V(t)$ ,  $u(0, 0) = 0$

$$\Rightarrow u(0, t) = \int_0^t V(\tau) d\tau = f(-ct)$$

$$\therefore f(-\xi) = \int_0^{\xi/c} V(\tau) d\tau \quad \xi \in (0, cT)$$

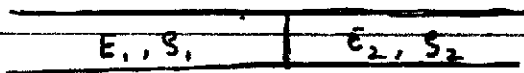
Similarly  $x = L$ ,  $t \in (t_1, t_1 + T)$   $t_1 = L/c$

$$g'(L+ct) + f'(L-ct) = 0 \quad \Rightarrow g$$

$$g(L+ct) = f(L-ct) = 0$$

Continue similarly for longer times.

### Example 3. Transmission problem



Consider a bar of uniform cross-section  $A$ , but made of heterogeneous materials,

$$E = \begin{cases} E_1 & x < 0 \\ E_2 & x > 0 \end{cases} \quad S = \begin{cases} S_1 & x < 0 \\ S_2 & x > 0 \end{cases}$$

Suppose a wave  $f(x-ct)$  travels through material 1 and ~~reaches~~ hits the ~~at~~ interface.

Part of it is transmitted  $h(x-c_2t)$  while part of it is reflected  $g(x+c_1t)$

To find  $g, h$ , we use the jump conditions

$$\llbracket v \rrbracket = 0 \quad \text{since the interface } x=0 \text{ does}$$

$$\llbracket T \rrbracket = 0 \quad \text{not move } (\dot{s}=0),$$

$$\therefore -c_1 f'(-c_1 t) + c_1 g'(c_1 t) = -c_2 h'(-c_2 t)$$

$$E_1 f'(-c_1 t) + E_1 g'(c_1 t) = E_2 h'(-c_2 t)$$

... solve for  $g'(c_1 t)$ ,  $h'(-c_2 t)$

use the initial data to

find  $g$ ,  $h$ .

In fact,

$$h'(-c_2 t) = \frac{E_1 c_2 + E_2 c_1}{2 E_1 c_1} f'(-c_1 t)$$

$$\text{and } g'(c_1 t) = \frac{E_2 c_1 - E_1 c_2}{2 E_1 c_1} h'(-c_2 t)$$

Note: If  $\boxed{E_1 c_2 = E_2 c_1}$ , then  $h'(-c_2 t) = f'(-c_1 t)$

and  $g'(c_1 t) = 0$

i.e., the wave is transmitted undistorted  
(but at a ~~different~~ different speed)  
with no reflection.

$$E_1 c_2 = E_2 c_1 \Leftrightarrow \frac{E_1}{c_1} = \frac{E_2}{c_2} \quad \left( \begin{array}{l} \text{impedance} \\ \text{matched} \end{array} \right)$$

$$\boxed{\frac{E}{c} = \rho c \dots \text{impedance}}$$